- (a) Starting construction;
- (b) placing backfill in the cutoff trench;
- (c) placing backfill around the primary spillway conduit or any other conduit that extends through the dam embankment and exits the downstream slope; and
- (d) starting any stage of construction not specified in this regulation for which the permit requires that the chief engineer shall be notified.

The contractor shall notify the Engineer of Record 72 hours prior to the required inspections to allow for the required 48 hours notice to the Chief Engineer.

(Authorized by K.S.A. 2006 Supp. 82a-303a; implementing K.S.A. 2006 Supp. 82a-301a and 82a-303a; effective May 18, 2007.)

K.A.R. 5-40-71. Inspection during dam construction, repair, and modification. (a) Except as specified in subsection (d), each high-impact dam shall be inspected by an engineer competent in the design of dams, or that engineer's authorized representative, at all times during any construction activity.

- (b) Each low-impact dam shall be inspected by an engineer qualified in the design of dams, or that engineer's authorized representative, whenever any of the following conditions is met:
- (1) Backfill is being placed in the cutoff trench of a dam.
- (2) Conduits and their appurtenances are being placed.
- (3) Backfill is being placed around a conduit.
- (4) Drain material and outlets are being installed.
- (5) Concrete forms and reinforcing steel are being placed.
- (6) Concrete is being placed.
- (7) Any other stage of construction required by the permit, approved plans, or approved specifications to be inspected occurs.
- (c) Before the start of construction, the permit holder shall provide the chief engineer in writing with the name, address, and telephone number of the engineer responsible for the inspection.
- (d) The inspecting engineer, or the engineer's authorized representative, shall not be required to be present during any of the following construction activities for a high-impact dam:
- (1) The clearing and grubbing of the construction site;
- (2) the removal of structures from the reservoir area other than the removal of a dam;
- (3) the installation of pollution-control measures, unless required by other authorities; (4) seeding;
- (5) mulching; and
- (6) the construction of a fence.
- (e) If the inspecting engineer, or the engineer's authorized representative, observes construction activity that is not in compliance with the approved permit, plans, or specifications and the contractor fails to correct the item or items that are not in compliance with the approved permit, plans, or specifications after being notified by the inspector, the inspector shall notify the chief engineer of the noncompliant activity.

(Authorized by K.S.A. 2006 Supp. 82a-303a; implementing K.S.A. 2006 Supp. 82a-301a and 82a-303a; effective May 18, 2007.)

LAKE OSHAWNO PRIMARY SPILLWAY REHABILITATION SPONSORED BY OSHAWNO LAKE ASSOCIATION OSAGE COUNTY, KANSAS DWR WSN: DOS-0055

DAM LOCATED IN THE NW $\frac{1}{4}$ OF NE $\frac{1}{4}$ OF SEC 01, T14S, R15E

NE CORNER SEC 01, T14S, R15E IN OSAGE COUNTY



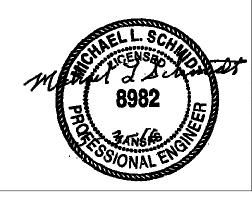
LAND OWNER
OSHAWNO LAKE ASSOCIATION INC.
P.O. BOX 417
CARBONDALE, KS 66414
OSAGE COUNTY, KS

CONSTRUCTION INGRESS / EGRESS

LOCATION MAP

SCALE: 1" = 1000'

SCHMIDT ENGINEERING CONSULTANTS, INC CIVIL, STRUCTURAL, AND ARCHITECTURAL ENGINEERING 311 COTTONWOOD ST STRONG CITY, KS 66869 620-273-8384



LLC 311 Collinated, Strong City, Karsis 8888 / 815 Carbon, Karsis 8888 / 820-343-

PROJ NO:

DRAWING:

DATE: Mar 18, 2015

DRAWN BY:KH GIRARDIN

CHK'D BY: ML SCHMIDT

PROJECT DESCRIPTION

This project is located south of the Osage / Shawnee County line in Osage County. The purpose of this project activity is to replace the Primary Spillway Pipe. The dam is being design to meet all current design standards except where appropriate waivers are granted by the Chief Engineer, State Board of Agriculture.

The CMP Primary Spillway Pipe that was installed 50 years ago has deteriorated. This pipe will be replaced with a PVC pipe that meets current design standards. In addition, the detention storage capacity of the reservoir and the auxiliary spillway are being modified to ensure long term protection of the dam. It is anticipated that the proposed maintenance actions will result in an improved structure and reservoir with the result that the effective life of the project will be extended another 40 to 50 years.

INDEX OF DRAWINGS:

- 1. TITLE SHEET
- 2. PROJECT DESCRIPTION, INDEX, AND DOWNSTREAM LANDOWNERS
- 3. DESIGN INFORMATION, TABLE OF QUANTITIES, AND STORAGE TABLE
- 4. DESIGN INFORMATION, DESIGN OF DAM SUPPLEMENT
- 5. SITE PLAN / DRAINAGE BASIN MAP
- 6. EXISTING PLAN
- 7. PROPOSED PLAN
- 8. PROPOSED PLAN / DEMOLITION PLAN
- 9. DAM AND AUXILIARY SPILLWAY PROFILES
- 10. CROSS SECTION / DRAINAGE DIAPHRAGM DETAILS
- 11. STILLING BASIN, CONCRETE PIPE SUPPORT DETAILS
- 12. PIPE DETAILS
- 13. TRASH RACK DETAILS
- 14. CONSTRUCTION AND SEEDING SPECIFICATIONS

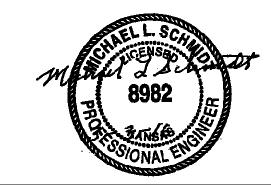
OSHAWNO LAKE ASSOCIATION PRIMARY SPILLWAY REHABILATION

NW ¼ OF, NE ¼ OF, NE ¼ OF SECTION 01, TOWNSHIP 14 S, RANGE 15 E

> DWR WSN: DOS-0055 OSAGE COUNTY, KANSAS

DOWNSTREAM LANDOWNERS

<u>NAME</u>	ADDRESS	CITY	STATE	<u>ZIP</u>
JANET JACKSON	449 SW COKER RD	WAKARUSA	KANSAS	66546
BURLINGTON NORTHERN SANTA FE RAILROAD (BNSF)	2650 LOU MENK DRIVE	FORT WORTH	TEXAS	76131



SCHMIDT Engineering Consultants, Inc.

CIVIL. STRUCTURAL. AND ARCHITECTURAL ENGINEERING
CONTOUROUS, Strong City, Norsis 6888 / 815 Johan 31, Empiria, Karss 6881 / 620-335-1332

SERVICES, LLC
SERVICES, LLC
CAD Professioner
CAD Profession Services

BY APP:	MLS			
ВУ	KHG			
DESCRIPTION	DWR REVIEW COMMENTS KHG MLS			
NO DATE:	6/15			
ON	1			

SHEET TITLE:
PROJECT DESCRIPTION
PROJECT:
PRIMARY SPILLWAY REHABILITA
OSHAWNO LAKE ASSOCIATION

REF: PROJ NO:

DATE: Mar. 18, 2015 DRAWN BY:KH GIRARDIN CHK'D BY: ML SCHMIDT

DRAWING:



TABLE OF QUANTITIES

UNITS	QUANTITY
LUMP SUM	1
ACRES	0.65
LUMP SUM	1
M GAL.	10
CU YD	7408
CU YD	71
CU YD	713
CU YD	1004
CU YD	112
TONS	17
CU YD	664
EACH	1
LIN. FT.	156
LIN. FT.	60
LIN. FT.	7
LIN. FT.	32
EACH	1
TONS	30
LUMP SUM	1
	LUMP SUM ACRES LUMP SUM M GAL. CU YD CU YD CU YD CU YD TUNS CU YD EACH LIN. FT. LIN. FT. LIN. FT. LIN. FT.

PIPE REPAIR INSTRUCTIONS

- 1. EXCAVATE AREA INDICATED, BE AWARE OF SAFE TRENCH STANDARDS.
- 2. REMOVE EXISTING TRASH RACK AND THE 30" Ø CONCRETE RISER.
- 3. REMOVE THE EXISTING PRIMARY SPILLWAY PIPE AND PIPE SUPPORTS.
- 4. BACKFILL AREA BY STANDARD COMPACTING METHODS TO THE PROPOSED PVC PRIMARY SPILLWAY PIPE GRADE.
- 5. INSTALL NEW 16" Ø PVC PRIMARY SPILLWAY PIPE, CONSTRUCT CONCRETE PIPE SUPPORT.
- 6. CONSTRUCT SAND DIAPHRAGM WITH DRAIN PIPE FOR NEW PVC PRIMARY SPILLWAY PIPE.
- 7. HAND COMPACT BACKFILL MATERIAL AROUND THE PRIMARY PIPE.
- 8. BACKFILL AREA BY STANDARD COMPACTING METHODS, THE PROJECT SPECIFICATIONS AND AS NOTED IN THE PROJECT DRAWINGS.
- 9. INSTALL TRASH RACK.
- 10. SEED AND MULCH DISTURBED AREA.
- 11. EXCAVATE STILLING BASIN, RIP RAP UPSTREAM EMBANKMENT AND THE STILLING BASIN WITH SALVAGED RIP RAP AND ADDITIONAL RIP RAP TO COMPLETE THIS ITEM.

NOTE: EMERGENCY ACTION PLAN (EAP) TO ADDRESS TEMPORARY EMERGENCY

SLOPE OF TOP OF DAM AND BACK BERM

top of dam.

The top of the back berm shall slope in the downstream direction with the inside edge of the berm 0.3' higher than the outside (downstream) edge of the berm. Design height shall be measured along the centerline of the top of the back berm.

TREE CLEARING

Trees within the outlet channel are to be removed or cut. Trees greater than 12" in diameter will be completely removed. The rootballs shall be removed and the area backfilled, compacted, and reseeded. Trees on the dam embankments shall be removed or cut flush to the ground and treated with an appropriate herbicide.

SLOPE PROTECTION

A protective cover of vegetation shall be established on all exposed surfaces of the structure and borrow area. Vegetation shall be per ITEM 11; SEEDING as noted in the Construction Specifications.

with a dark shade of "latex" paint as recommended by the manufacturer (or by encasing in a protective material). The Primary Spillway pipe shall be painted, the 4 inch drain pipes shall be encased in a CMP as noted in the drawings.

The top of the dam shall slope so that the upstream edge of the top of dam is 0.3' lower than the downstream edge of the top of dam. Design height shall be measured along the centerline of the

PVC PROTECTION FROM SUNLIGHT

Each portion of polyvinyl chloride pipe that is exposed to sunlight shall be protected by painting

_, AND ARCHITECTURAL ENGINEERING 6888 / 815 Graham St., Emporia, Kansas 66801 / 620-343-Engineering (STRUCTURAL. I, Strong City, Kansas 6

11/25/21/20						
STW	BY APP:	MLS	MLS	MLS	MLS	
БНХ	АВ	KHG	KHG	KHG	KHG	
\$ 03/16 DWR REVIEW COMMENTS KHG MLS	DESCRIPTION	DWR REVIEW COMMENTS KHG MLS	2 12/15 DWR REVIEW COMMENTS KHG MLS	3 01/16 DWR REVIEW COMMENTS KHG MLS	4 02/16 DWR REVIEW COMMENTS KHG MLS	REVISIONS
03/16	NO DATE:	1 9/15	12/15	01/16	02/16	
RD	9	1	CJ.	3	4	
		بنا				

STORAGE \ QUANTITIES

REHABILITATION

SPILLWAY REHABILI LAKE ASSOCIATION 0 f PRIMARY SOSHAWND TABLE

REF: PROJ NO:

DATE: Mar. 18, 2015 DRAWN BY:KH GIRARDIN CHK'D BY: ML SCHMIDT

DRAWING:

DRAW DOWN OF THE LAKE BY SIPHON OR PORTABLE PUMP.

TOP OF DAM ELEVATIONS

STATION	EXISTING	PLANNED	CUT/FILL_	SETTLEMENT	NOTES
0+00	1012.00	1012.00	0.0	0.0	DUTSIDE A.S.
0+18	1007.00	1006.00	-1.0	0.0	INSIDE A.S
0+38	1007.00	1006.00	-1.0	0.0	CL A.S
0+58	1007.10	1006.10	-1.0	0.0	INSIDE A.S
0+82	1012.40	1012.40	2.6	0.0	END OF DAM
1+22	1013.00	1015.00	2.0	0.0	@ WEST SIGN
1+89	1014.60	1015.00	0.5	0.0	P.S. LOCATION
2+89	1015.70	1015.00	0.0	0.0	
3+89	1015.80	1015.00	0.0	0.0	
4+89	1014.30	1015.00	0.7	0.0	
5+50	1014.20	1015.00	8.0	0.0	END OF DAM (EXISTIN
6+30	1014.00	1015.00	1.0	0.0	END OF DAM (NEW)

Note: The original dam was constructed in approximately 1957. Therefore, settlement is designed only for that portion of the dam which will be disturbed by repairs and any areas needing added settlement as a result of the increase in design dam height.

TABLE OF DISCHARGES

Elevation MLS (feet)	Pipe Flow (cfs)	Aux. Spillway Flow (cfs)	Outlet Elev. MLS
1001	0.0	0.00	985.2
1002	17.8	0.00	985.2
1003	18.3	0.00	985.2
1004	18.8	0.00	985.2
1005	19.2	0.00	985.2
1006	19.7	0.00	985.2
1007	20.2	0.00	985.2
1008	20.6	136.00	985.2
1009	21.0	384.00	985.2
1010	21.5	705.00	985.2
1011	21.9	1085.00	985.2
1012	22.3	1516.00	985.2

STORAGE TABLE

Elevation MLS (feet)	Surface area (acres)	Incremental volume (ac-ft)	Accumulative volume (ac-ft)
974	0.0	0.00	0.00
976	0.26	0.26	0.26
980	1.08	2.68	2.94
984	2.88	7.92	10.86
988	5.16	16.08	26.94
992	6.69	23.70	50.64
996	8.71	30.80	81.44
1000	11.69	40.80	122.24
1001	12.17	11.93	134.17
1004	14.00	39.26	173.43
1007	15.80	44.70	218.13
1008	16.50	16.15	234,28
1012	20.30	73.60	307.88
1013	21.50	20.90	328.78
1014	23.80	22.65	351.43
1015	26.30	25.05	376.48
1016	28.80	27.55	404.03

SCHMIONIC, Inc. STRUCTURAL. AND ARCHITECTURAL ENGINEERING COMMINGS 6000 / 815 Orbin St. Empiric, Mass 6001 / 600-34-000.



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MLS	APP:	STW		MLS	MLS	STW	
KHG	ВУ	KHG MLS		KHG MLS	KHG MLS	KHG	
ADDED ITEM #12 RIP RAP KHG MLS	DESCRIPTION	DWR REVIEW COMMENTS		DWR REVIEW CUMMENIS	3 01/16 DWR REVIEW COMMENTS	4 02/16 DWR REVIEW COMMENTS KHG MLS	REVISIONS
3 3/16	NO DATE:	1 9/15		2 12/15	01/16	02/16	
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See St. 1986

SHEET TITE:

DESIGN OF DAM SUPPLEMENT

PROJECT:

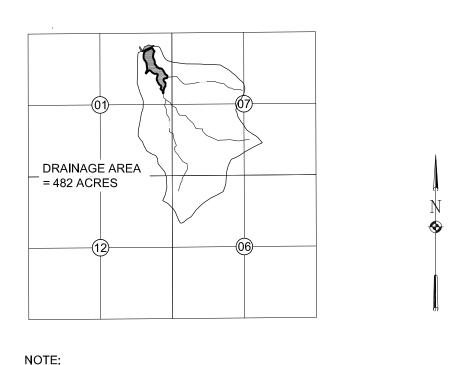
PRO

DATE: Mar. 18, 2015
DRAWN BY:KH GIRARDIN
CHK'D BY: ML SCHMIDT

DRAWING:

4

PLAN SET.DWG



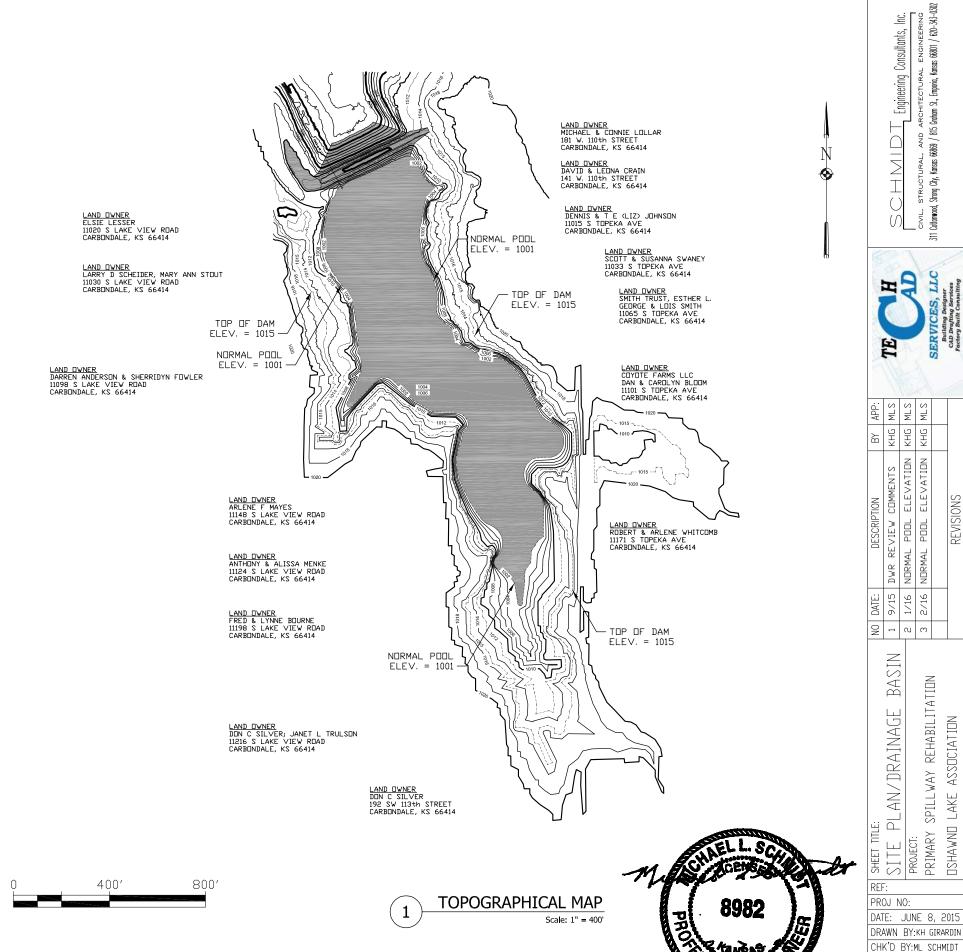
4000′

8000'

DRAINAGE AREA MAP Scale: 1" = 4000'

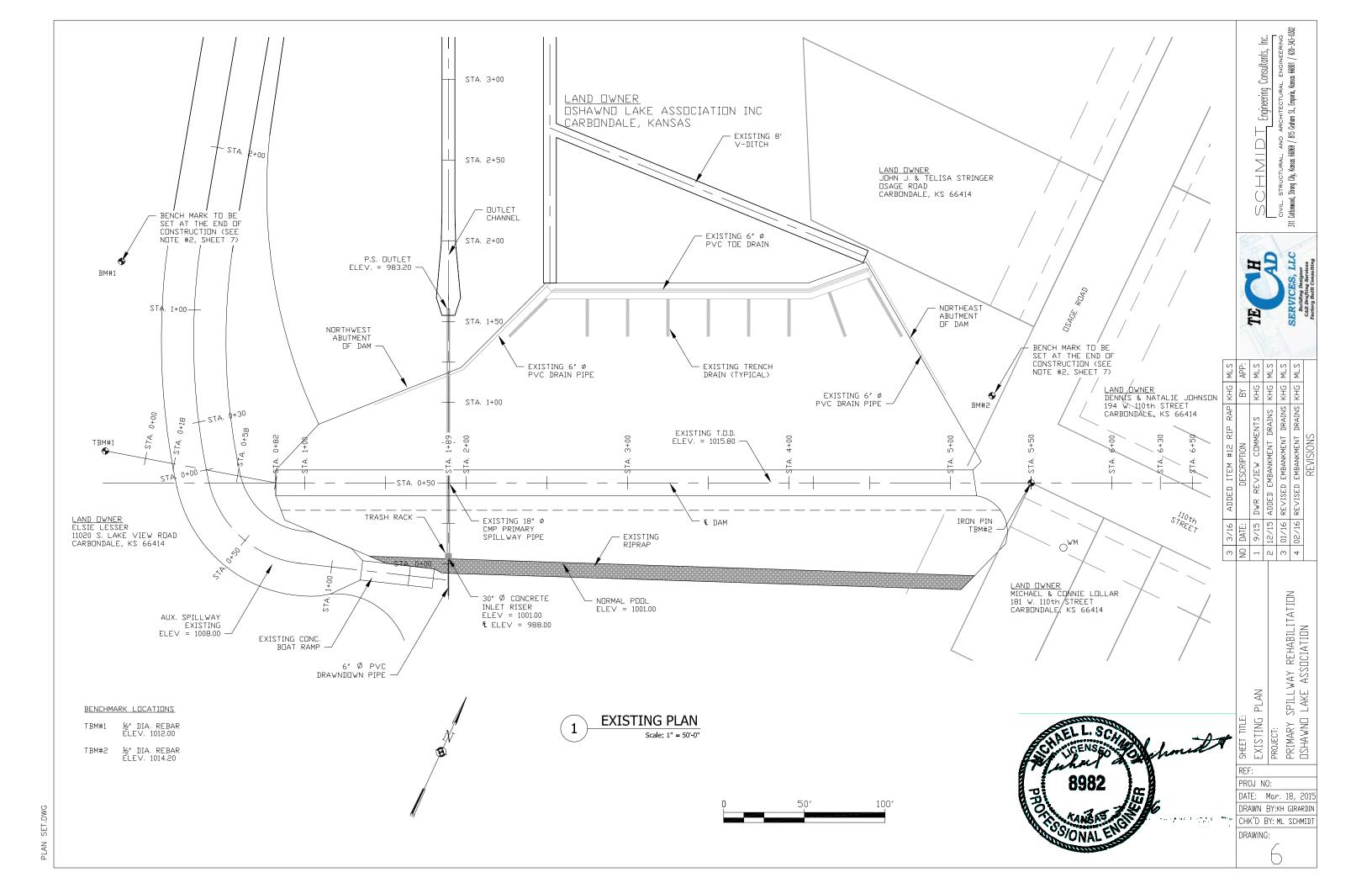
DRAINAGE AREA = 482 ACRES

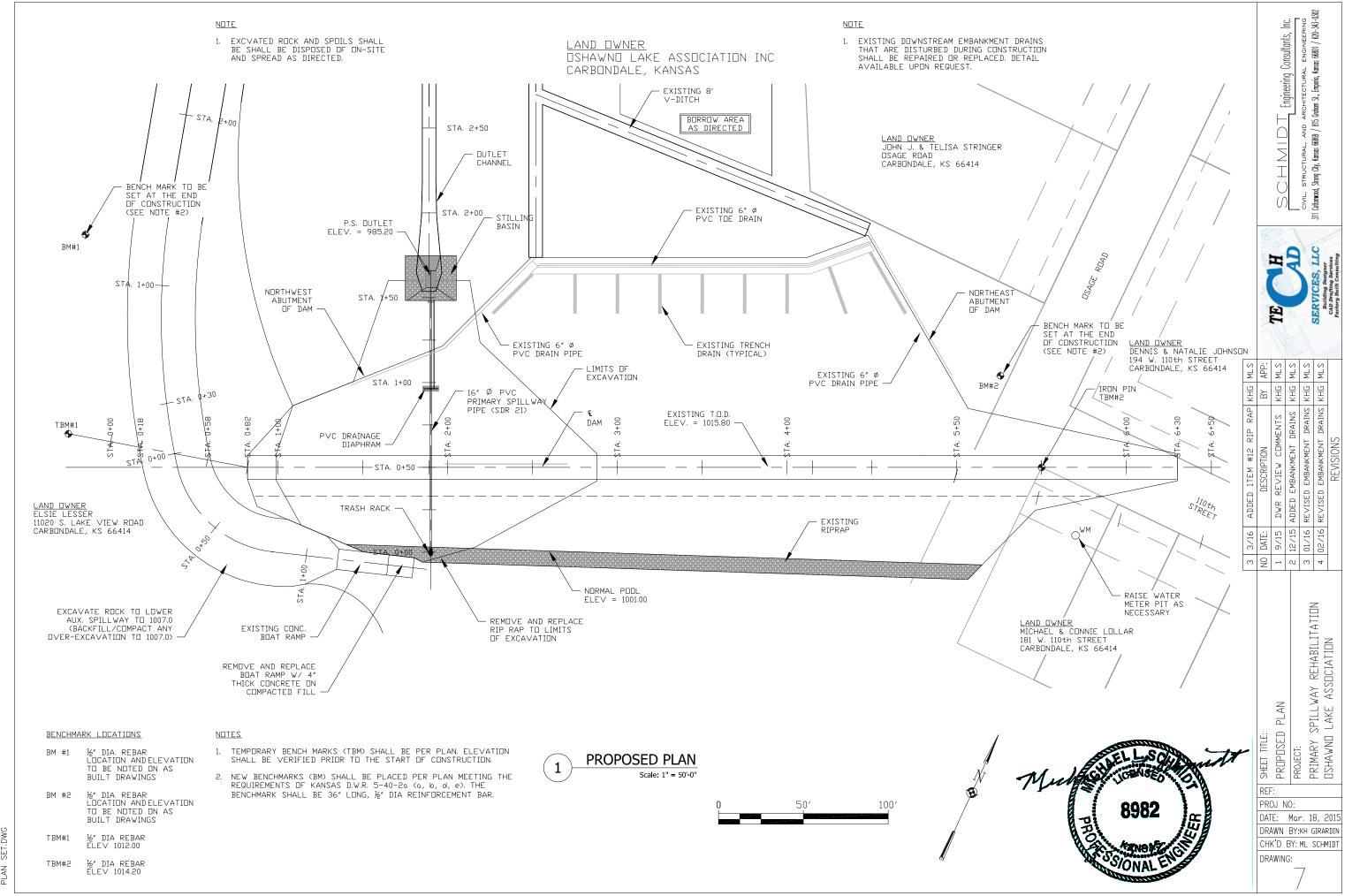
= 0.75 SQUARE MILES AS DWR

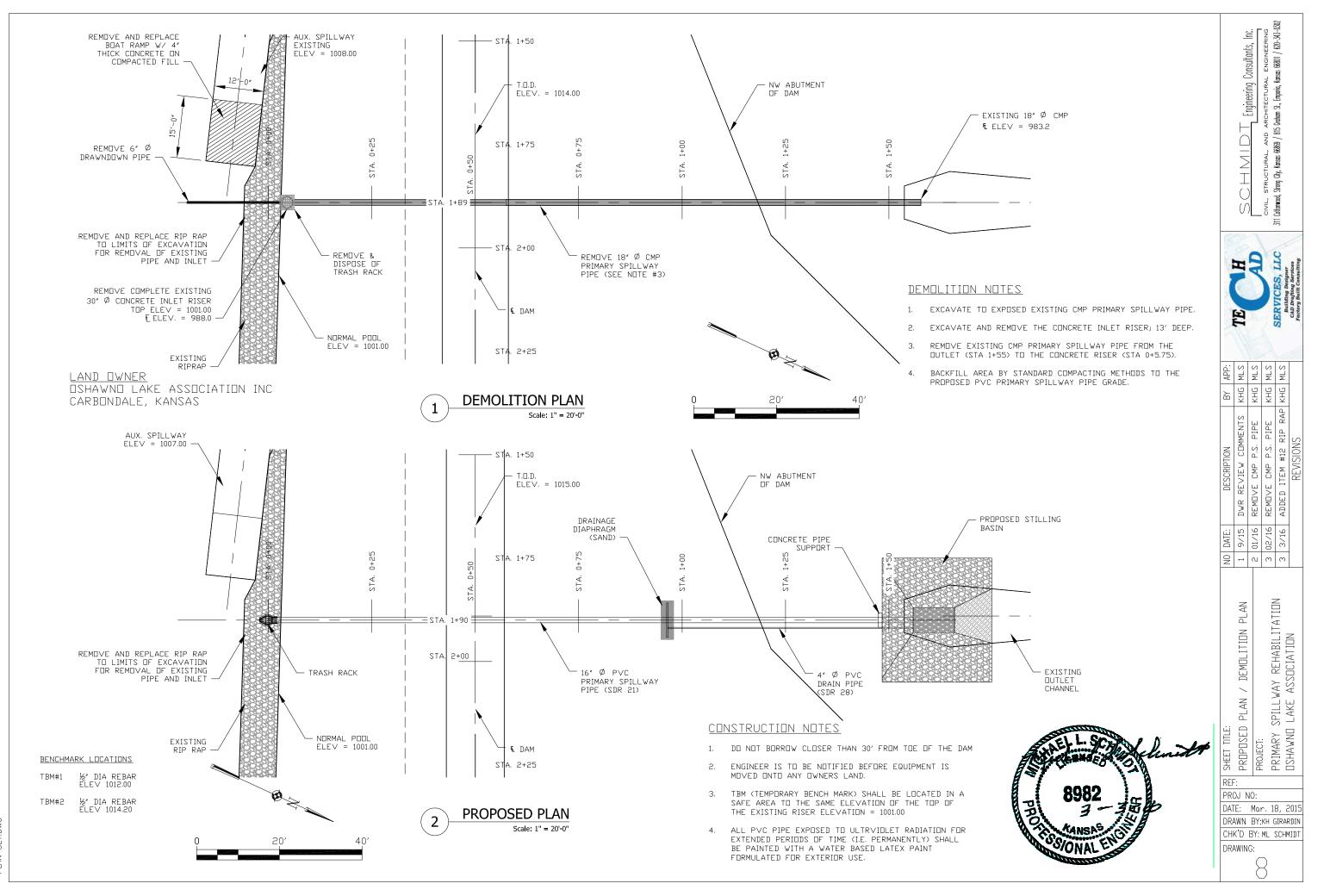


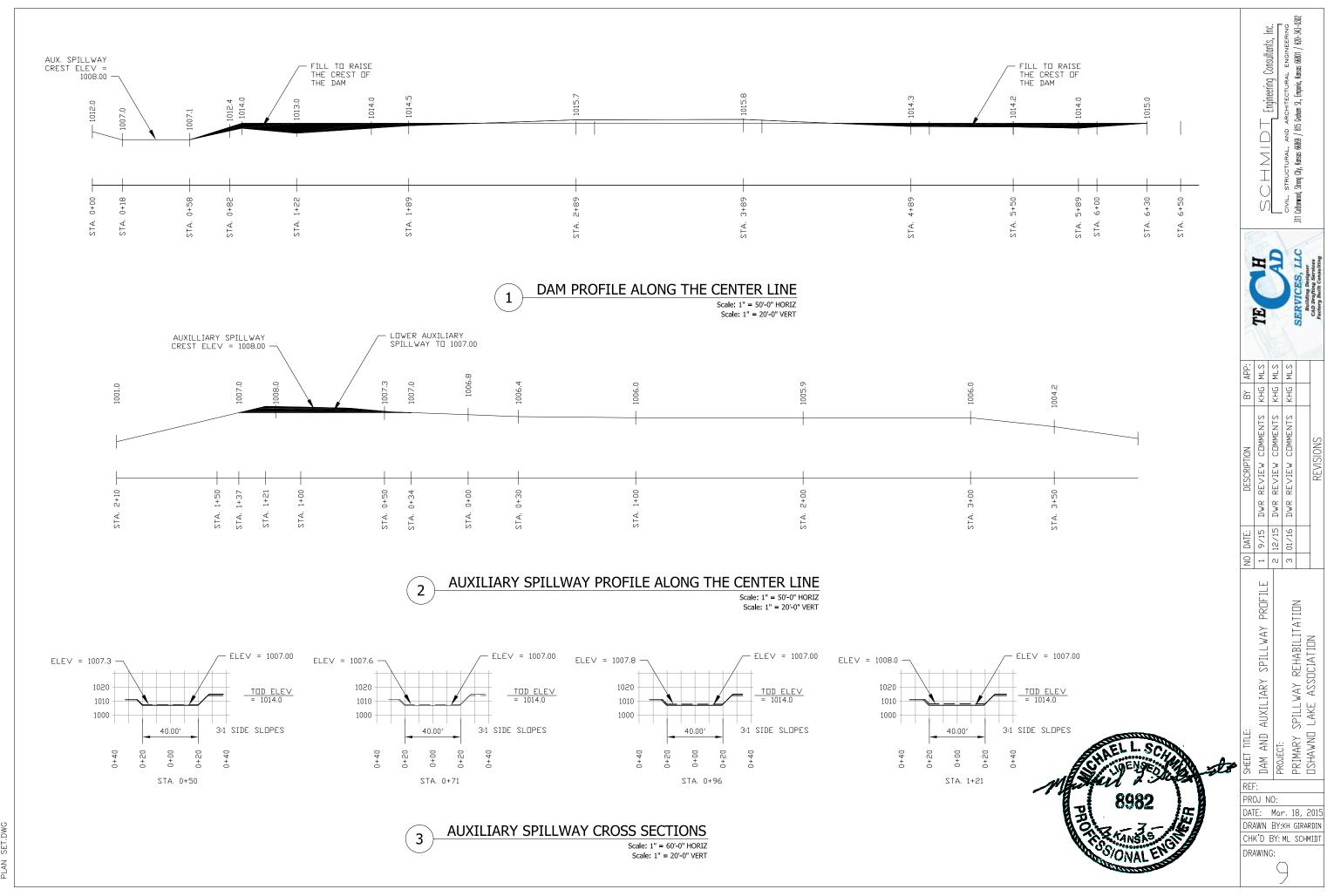
ASSOCIATION

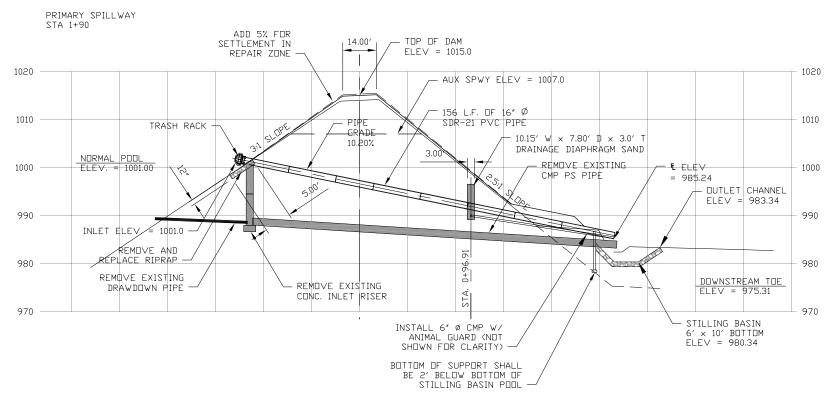
DRAWING:

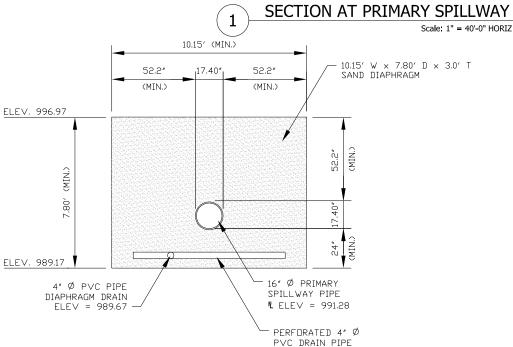












3 PVC SAND DIAPHRAM DETAIL Scale: 1" = 5'-0"

NOTES:

- INSTALL 6" CMP DUTLET PROTECTION ON LAST 16' OF 4" PVC DUTLET. USE ANIMAL GUARD ON DUTLET.
- USE CLAY PLUGS 3' LONG BELOW 4" PVC ELBOW BELOW DRAINAGE DIAPHRAGM AND ABOVE PLACEMENT OF 6" CMP.
- 3. BACKFILL OVER 4" PVC PIPE SHALL INCLUDE 3" BELOW PIPE & 6" OVER PIPE.

NOTES:

- INSTALL 60 LF 4" Ø SDR-28 PVC DUTLET PIPE FOR NEW PVC PRIMARY SPILLWAY PIPE DRAINAGE DIAPHRAGM.
- 2. 4" DRAINAGE PIPE SHALL EXTEND FROM THE DRAINAGE
 DIAPHRAGM TO STILLING BASIN. THE 4" PVC PIPE IS PLACED
 ALONG THE SIDE AND PARALLEL WITH THE PRIMARY SPILLWAY
 PIPE. EXIT ELEVATION TO BE AT 985.0 OR HIGHER.

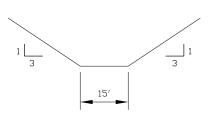
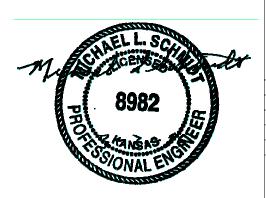




TABLE OF PIPE GRADES								
PIPE STATION	P.S. GRADE	ELEV.	4″ Ø GRADE	ELEV.				
0+00.69		1001.00						
0+40.00	10.20	996.99						
0+80.00	10.20	992.91						
0+96.91	10.20	991.18		989.67				
1+20.00	10.20	988.83	7.67	987.90				
1+55.20	10.20	985.24	7.67	985,20				

FINE DRAINFILL							
GRAD,	ATION						
SIEVE	% PASSING						
3/8	100						
4	95 - 100						
8	80 - 100						
16	50 - 85						
30	25 - 60						
50	10 - 30						
100	2 -10						
200	0 -5						



CIVIL, STRUCTURAL, AND ARCHITECTURAL ENGINEERING
311 Cutumood, Stong City, Korss 6889 / 815 Cutum St, Empoin, Korss 6890 / 620-343-

TE AD
SERVICES, LLC
Builting Designer

			v		20		
MLS	BY APP:	KHG MLS	2	MLS	MLS	KHG MLS	
ХHG	ВУ	KHG	71.17	חח	KHG MLS	KHG	
ADDED ITEM #12 RIP RAP KHG MLS	DESCRIPTION	DWR REVIEW COMMENTS	STINIMADO / VIII/ VIO O/ VG	DWR REVIEW COMMENIS NAG MLS	DWR REVIEW COMMENTS	REMOVE CMP P.S. PIPE	REVISIONS
3 3/16	NO DATE:	1 9/15	7 /15	C //13	3 12/15	4 01/16	
ო	9	1	r	U	т	4	

CTION / DRAINAGE DIAPHRAM
SPILLWAY REHABILITATION
LAKE ASSOCIATION

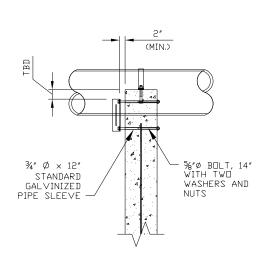
SHEET TILE:
CRDSS SECTION / I
PROJECT:
PRIMARY SPILLWAY
OSHAWNO LAKE AS:

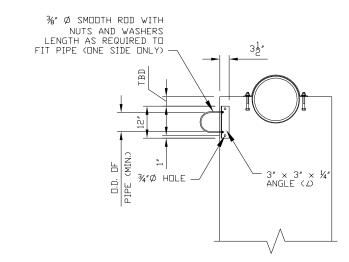
REF: PROJ NO:

DATE: Mar. 18, 2015
DRAWN BY:KH GIRARDIN
CHK'D BY: ML SCHMIDT

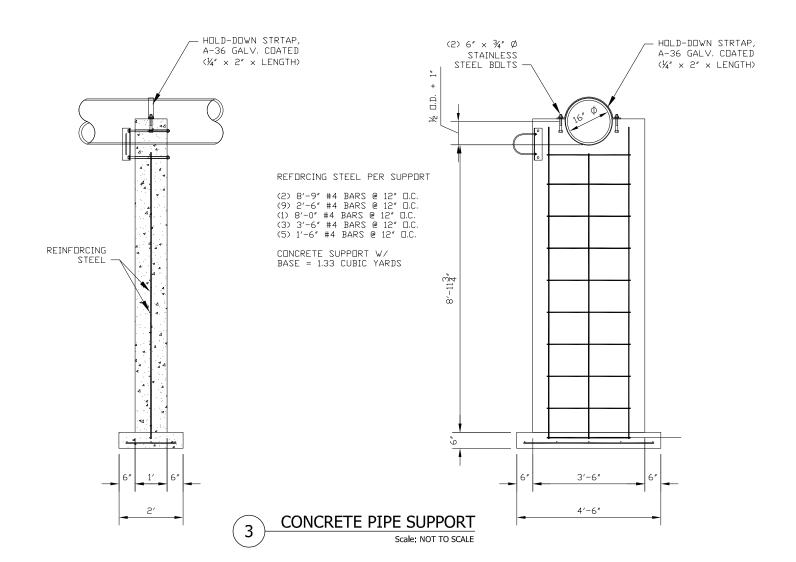
DRAWING:

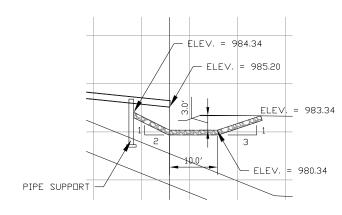
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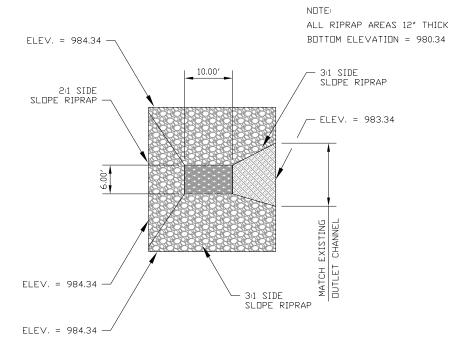


4 PIPE SUPPORT BRACKETS Scale: NOT TO SCALE

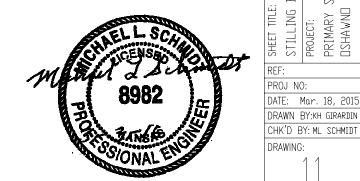




2 STILLING BASIN SIDE VIEW Scale: NOT TO SCALE

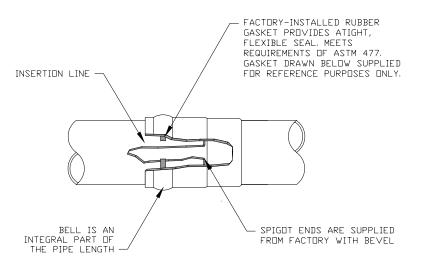


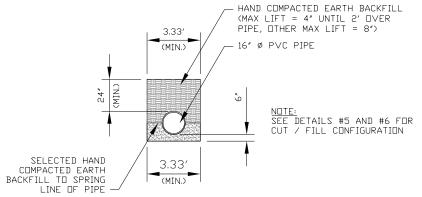




SCHMIDT Engineering Consultants, Inc.	INIL, STRUCTURAL, AND ARCHITECTURAL ENGINEERING 311 Odiomood, Sing Ciy, Karss 6088 / 815 Gabra S., Enpori, Karss 6601 / 620-343-0302

SUPPORT SPILLWAY REHABILITATION LAKE ASSOCIATION SHEET TITLE:
STILLING BASIN/CONCRETE PIPE SU
PROJECT:
PRIMARY SPILLWAY REHABILITATII
DSHAWND LAKE ASSOCIATION





BACKFILL DETAIL

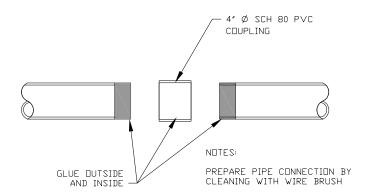
Scale: NOT TO SCALE

CONTRACTOR MAY EXCAVATE TRENCH WIDTH WIDER, AT HIS EXPENSE.

16" Ø 12.87 CU. FT./FT OF EARTH FILL HAND COMPACTION.

PIPE SHALL BE LAID ON COMPACTED SOIL

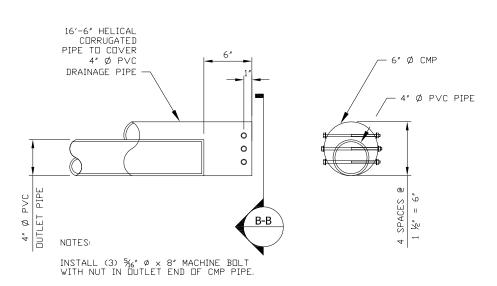
BELL AND SPIGOT CONNECTION Scale: NOT TO SCALE



4" PIPE JOINT CONNECTION

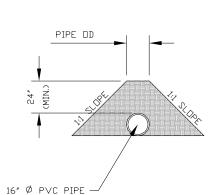
NOTES:

FURNISH AND PLACE PVC PIPE MATERIAL ACCORDING TO THE FOLLOWING A.S.T.M. D1784-PVC COMPOUNDS, D1785 PVC SCHEDULE RATED PIPE, D2564-PVC SOLVENT CEMENT, D2564-PVC EXTRERNAL LOADING PROPERTIES AND D2444-PVC IMPACT RESISTANCE OF THERMO PLASTIC PIPE.



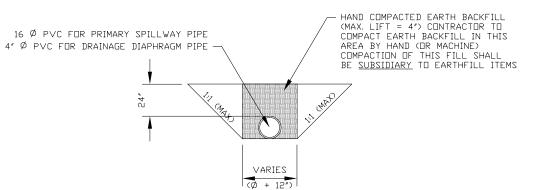
ANIMAL GUARD

Scale: NOT TO SCALE



EARTH BACKFILL





HAND COMPACTION DETAIL 6 Scale: NOT TO SCALE

KEEP ALL EARTH-MOVING EQUIPMENT A MIN. 2' FROM PIPE. INSTALL AND VIBRATE SELECTED BACKFILL MATERIAL AFTER INSTALLATION OF PIPE. SELECTED SOILS FOR HAND COMPACTION TO BE APPROVED

NOTE:

CMP PIPE EXCAVATION TRENCH Scale: N.T.S.

L, AND ARCHITECTURAL ENGINEERING 66869 / 815 Gaham SI, Emporia, Kansas 66801 / 620-343-Engineering (V RAL STRUCTL , Strong City, K

SPILLWAY REHABILITATION LAKE ASSOCIATION

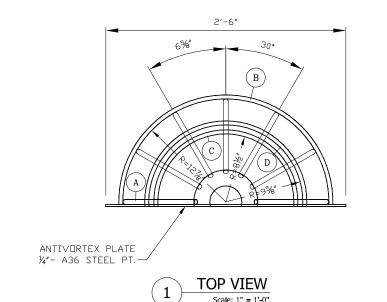
DETAILS PROJECT:
PRIMARY S

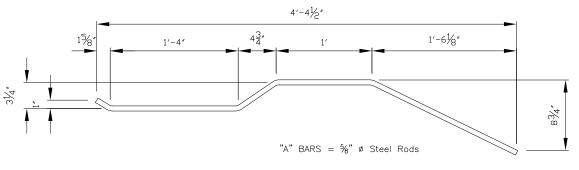
DSHAWND 1 SHEET '

REF: PROJ NO:

DATE: June 1, 2015 DRAWN BY:KH GIRARDIN CHK'D BY: ML SCHMIDT

DRAWING:





BENDING DIAGRAM "A" RODS Scale: 1" = 1'-0"

"B" BARS = $\frac{1}{2}$ " \emptyset Steel Rods -ANTIVORTEX PLATE 1/4"- A36 STEEL PT. BENDING DIAGRAM "B" RODS Scale: 1" = 1'-0"



"C" BARS = ½" Ø Steel Rods "D" BARS = ½" Ø Steel Rods

BENDING DIAGRAM "C" AND "D" RODS Scale: 1" = 1'-0"

MATERIAL LIST AND FABRICATION INSTRUCTIONS MATERIAL TYPE LENGTH & TOTALS
AND QUANTITY OR WIDTH LENGTH WT PART DESCRIPTION ROUND BARS Ø QTY. WT. EACH NEXT FT. OR # 1.04 4'-713/16 ″B″ R□D ½" 4 0.67 3'-6¹1/16" 10 "C" ROI 0.67 4'-8%6" 0.67 5'-3%" THICK QTY. WT. ANTI-VORTEX PLATE 1/4" 7.3 10.21 #'S/SQ. FT. 2'-6" X 2'-11" *64.45 TANK LUG *DDES NOT INCLUDE WASTE ALL BARS AND PLATES TO BE TYPE A/36 STEEL

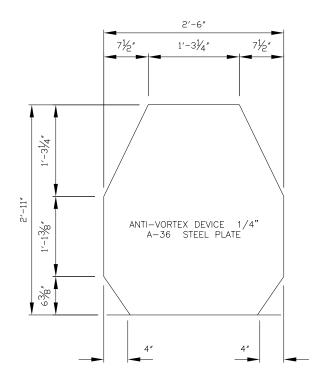
ADD 1 BAR WIDTH PER 90 DEGREE OF BENDS TO LENGTH

BAR LENGTHS ARE FROM CENTER LINE OF BAR AT BEND

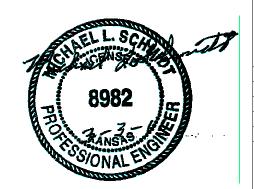
TOTAL UNIT WEIGHT AFTER FABRICATION= 150 +/- LBS. (UNCOATED)

WELD ALL CONNECTION POINT OF BARS WITH ALL ROUND BUTT WELD. MIN. 1" TACK WELD ON 6" CENTERS VORTEX PL. TO BARS.

CHIP AND WIRE BRUSH ALL WELDS. REMOVE ALL RUST FROM FABRICATED UNIT AND HOT DIP GALVANIZED AS PER ASTM A123, EXCEPT HOLD DOWN RINGS.



ANTI-VORTEX PLATE DETAIL



L. AND ARCHITECTURAL ENGINEERING 68869 / 815 Gaham St., Emponi, Karsas 68801 / 620-343-0302 Engineering (ONIL, STRUCTURAL, Cottonnood, Strong City, Kansas &

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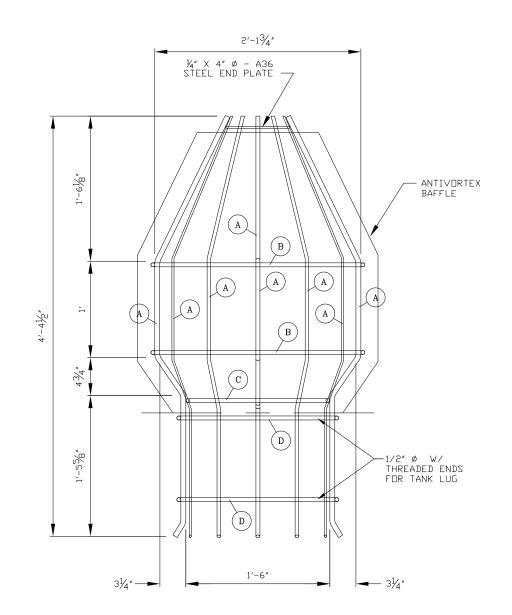
SPILLWAY REHABILITATION LAKE ASSDCIATION

DETAILS TRASH RACK PROJECT:
PRIMARY S
DSHAWND

REF: PROJ NO:

DATE: Mar. 18, 2015 DRAWN BY:KH GIRARDIN CHK'D BY: ML SCHMIDT

DRAWING:



SIDE VIEW Scale: 1" = 1'-0"

CONSTRUCTION SPECIFICATIONS

1. SCOPE:

The work shall consist of all construction operations and furnishing all materials as required by the drawings and specifications for the complete installation of this project.

2. LOCATION:

The location of the embankment, auxiliary spillway, and appurtenances shall be as specified on the drawings.

3. SITE PREPARATION

<u>Foundation Area:</u> The foundation area will be thoroughly scarified to a minimum depth of 6 inched, before placement of fill material and any needed moisture is added, to insure that the first layer of fill material can be bonded to the foundation. To facilitate compaction equipment and to provide safety, all channel banks and breaks shall be sloped no steeper the 2 horizontal to 1 vertical.

4. EXCAVATION:

To the extent they are suitable and approved by the inspector, excavated materials are to be used as fill materials.

<u>Cutoff and Primary Spillway Trenches:</u> These trenches shall be excavated to the lines, grades, and width shown on the drawings, or as specified by the inspector for a depth adjustment during excavation. The trenches shall be kept free of standing water during backfill operations. Backfill shall be made with selected impervious material, approved by the inspector, and will be placed in the same manner as specified for earth fill.

Auxiliary Spillway and Outlet Channel: These excavations shall confirm to the lines, grades, bottom width, and side slopes shown on the drawings or as staked in the field.

<u>Borrow:</u> The location of the borrow area shall be as determined by the landowner. Borrow pits wall be excavated and maintained in a manner to eliminate steep and unstable side slopes, or other hazardous condition. Side slopes shall be no steeper than 3:1.

5. PRIMARY SPILLWAY:

The materials and manufacture of the pipe, drainage diaphragms, coupling bands, coatings, and other appurtenances shall be as shown on the drawings, and shall confirm to the appropriate federal or ASTM specifications suitable for the intended purpose. The pipe shall be laid to the line and grades shown on the drawings, be placed on properly compacted earth fill, and be uniformly bedded to the depth and in the manner specified. A static pressure test is not required for PVC pipe with the proper water-tight couplings. Selected, impervious backfill material shall be placed around the conduit and appurtenances in layers not more than 4 inches thick before compaction and each layer shall be thoroughly compacted by hand tamping, manually directed power tampers, or plate vibrators to the density of the surrounding material. The height of the fill shall be increased at approximately the same rate on all sides of the structure. Heavy equipment shall not be operated within 2 feet of any structure.

6. PLACEMENT OF EARTHFILL

The material placed in the fill shall be free of sod, roots, frozen soil, stones over 3 inched in diameter. The placing and spreading of fill material shall be started at the lowest point of the foundation and the fill brought up in horizontal layers of such thickness that the required compaction can be obtained. The fill shall be constructed in continuous, horizontal layers, except where openings or sectionalized fills are called for. In those cases the slope of the bonding surfaces between the embankment-in-place and the embankment to be placed will not be steeper than 3:1. The distribution and gradation of materials shall be such that there will be no lenses, pockets, streaks, or layers of material differing substantially in texture or gradation from the surrounding material. Stockpiled topsoil strippings will be placed on the outer portion of the embankment as a part of each lift. Topsoil shall not be less than 6 inches nor more than 2 feet thick, measured vertically, and shall be compacted concurrently with the earth fill. The completed work shall conform to the lines, grades, and elevations shown on the drawings, or as staked in the filed.

7. MOISTURE CONTROL:

The moisture content of the fill material shall be such that the required compaction can be obtained. Material that is too wet shall be dried to meet the requirement, and material that is too dry shall have water added and mixed until the requirement is met.

The contractor shall engage the services of a qualified soils testing firm, the soils testing firm shall obtain samples of proposed fill soil and provide Standard Proctor Moisture/Density (ASTM D698) curves for the material and make recommendations as to the moisture content. The recommended moisture content shall be no less than 2% below nor 5% above optimum moisture, unless recommended differently by the soils firm.

8. COMPACTION:

The earth fill materials shall be placed in layers not exceeding 9 inches before compaction. Construction equipment shall be operated over the areas of each lift of earth fill in a way that will result in the required compaction. Special compaction equipment will be used when the required compaction cannot be obtained without it.

The moisture content at the time of compaction shall be within limits required to:

- a. Prevent bulking of material under action of the grading or compacting material.
- b. Prevent the adherence of the fill material to the treads and tracks of the equipment.
- c. Insure the crushing and blending of the soil clods and aggregations into a reasonably homogeneous mass.

The soils testing firm shall have a qualified technician on site during the back filling operations for fill in the dam structure. The technician shall test compaction and moisture content of the fill material at the time of back filling to insure requirements are met. Fill soil placed in the dam structure shall be compacted to 90% of Standard Proctor (minimum) at the required moisture content, unless recommended differently by the soils firm.

9. SUPPLEMENTARY CONDITIONS:

Per K.S.A.82a-303a, further defined in Division of the Water Resources rules and regulations, the followings conditions apply:

<u>Landowner Duties:</u> The landowner shall provide adequate supervision and inspection of the project during the period of construction in accordance with all requirements and conditions of the permit to construct.

<u>Contractor Duties:</u>

- a. It is required that the primary spillway and appurtenances, including fill material around the pipe, be inspected and approved by a representative of DWR prior to placing fill material over the pipe.
- o. It is required that no construction shall proceed on the project until each respective DWR requirement is complied with.
- c. It is required that the contractor shall be responsible for notifying the supervisory inspector and DWR 48 hours in advance per K.A.R. 5-40-70:
- l) Commencement of construction on the project.
- Placement of any material on any portion of the foundation.
- 3) Installation of the primary spillway pipe and appurtenances.

10. <u>DEWATERING</u>

Excavations through the dam shall be dewatered as required to maintain the excavations free of water and to keep the groundwater surface sufficiently below the bottom of the excavations so as not to cause settlement, pumping, subsidence, etc. Dewatering shall be achieved by lowering the Lake level sufficiently below the upper level of the new primary spillway pipe to control the ground water level, IF approved by the Dwner. If sufficient dewatering of the excavations cannot be achieved by lowering the lake level, a ground water pumping system shall be used. The system shall be designed by a Kansas licensed Geologist or Geotechnical Engineer and the system design shall be submitted to the Project Engineer for review. Discharge of the water from the dewatering system shall comply with EPA and KDHE regulations and requirements. The contractor shall be responsible for the cost of and all issues relating to all dewatering operations, system design, and adverse results of dewatering operations, permitting, and discharge requirements, water disposal, etc.

11. SEEDING:

- a. <u>SCOPE:</u> A protective cover of vegetation shall be established on all exposed surfaces of the structure and borrow area.
- b. <u>SEEDBED PREPARATION</u>: The area to be seeded will be reasonably smooth with all washed and gullies filled to confirm to the desired cross section. All debris, such as wood, stones, or other objects that will interfere with the mowing or seeding will be removed from the area. Prior to seeding, the entire area will be "roughened" by at least two passes with a spike-tooth harrow. Additional tillage may be required.
- spike-tooth harrow. Additional tillage may be required.

 c. METHOD OF SEEDING: Drilling: Grass seed will be planted with a grass drill equipped with double coulter furrow openers with depth bands. Seed shall be planted ¼ to ¾ inch deep. Broadcasting: On small seeding projects (generally less than 3 acres), and on areas not safely accessible, broadcasting may be used. When broadcasting is used, at least one pass with a spike-tooth harrow, or equivalent operations, will be made over the seeded area to cover the seed.
- d. <u>SEED MIXTURE: Upland Mixture:</u> The grass seed mixture used on the dam embankment and auxiliary spillway shall be applied at a minimum rate of 16 PLS pounds per acre. The following composition should be used:

Western Wheatgrass (Barton)	6.0 PLS pounds
Smooth Brome	5.0 PLS pounds
Kentucky Fescue	5.0 PLS pounds

12. <u>RIP RAP:</u>

Stone for Rip Rap shall be crushed linestone free of earth, chert, cracks, seams, soapstone, and shale or other easily disintegrated material. Size shall be as follows.

Sieve Size:	Percent Retained on Sieve Size:
12"	0
9″	20 / 40
6″	30 / 70
4 "	65 / 85
3″	80 / 100
2"	90 / 100

Stone shall be placed so that the spaces between larger stones are filled with smaller stones to achieve full coverage of subgrade. The surface of the rip rap shall be rammed and compacted to obtain as tight a surface as practical for the gradation, and to conform to the lines, grades, and slopes on the drawings.

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	MLS	KHG	EM #12 RIP RAP	ADDED IT	3 3/16 ADDED ITEM #12 RIP RAP KHG MLS
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	MLS	KHG	DWR REVIEW COMMENTS KHG MLS		1 9/15 DWR RE
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TIDN SPECIFICATIONS
SPILLWAY REHABILITATION
LAKE ASSOCIATION

SHEEL HILE:
CONSTRUCTION SPE
PROJECT:
PRIMARY SPILLWA
OSHAWNO LAKE A3

REF: PROJ NO:

DATE: Mar. 18, 2015 DRAWN BY:KH GIRARDIN CHK'D BY: ML SCHMIDT

DRAWING:

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